# NORDIC STRUCTURES

## ES12 Columns - Design Properties

#### ES12 Columns – Design Properties

Application	Columns				
Axonometry	X Y X X				
Appearance grade	Industrial				
Stress grade	ES12				
Layup combination	ES12/NPG				
Bending about X-X axis					
Bending moment, f <sub>bx</sub> <sup>(a)</sup>	4,453 psi				
Longitudinal shear, f <sub>vx</sub> <sup>(b)</sup>	363 psi				
Compression perpendicular to grain, f <sub>cpx</sub> <sup>(b)</sup>	1,088 psi				
Shear-free modulus of elasticity, E <sub>x</sub>	1,900,000 psi				
Apparent modulus of elasticity, E <sub>x,app</sub> <sup>(d)</sup>	1,800,000 psi				
Bending about Y-Y axis					
Bending moment, f <sub>by</sub> <sup>(a)</sup>	4,453 psi				
Longitudinal shear, f <sub>vy</sub> <sup>(b)</sup>	363 psi				
Compression perpendicular to grain, f <sub>cpy</sub> <sup>(c)</sup>	1,088 psi				
Shear-free modulus of elasticity, E <sub>y</sub>	1,900,000 psi				
Apparent modulus of elasticity, E <sub>y,app</sub> <sup>(d)</sup>	1,800,000 psi				
Axially loaded					
Compression parallel to grain, $f_c$	4,786 psi				
for 3 laminations	3,539 psi				
Tension parallel to grain, f <sub>t</sub>	2,959 psi				
Tension perpendicular to grain, f <sub>tp</sub>	74 psi				
Modulus of elasticity, E <sub>a</sub>	1,900,000 psi				
Mean relative density, G <sup>(e)</sup>	0.47 -				
Density (for member weight), $\rho^{(f)}$	35 pcf				

a) The size factor for bending, K<sub>2bg</sub>, shall be calculated as per Clause 7.5.6.5.1 of CSA 086:19, where the beam width, b, is taken as the full member width.

b) The specified fracture shear strength at a notch, f<sub>r</sub>, shall be calculated as per Clause 7.5.7.5.2 of CSA O86:19, where the effective lamination width, b<sub>eff</sub>, is taken as the full member width.

c) The size factor for bearing, K<sub>zcp</sub>, shall be calculated as per Clause 6.5.6.4 of CSA O86:19, where the width and the depth are respectively the full member width and the thickness of lamination.

d) The apparent modulus of elasticity values include a 5% shear deflection. For column stability calculations, E<sub>05</sub> shall be determined by multiplying the tabulated apparent modulus of elasticity by 0.87.

e) Mean relative density values, G, for dowel-type fastener design in accordance with CSA O86.

f) Density values, ρ, for a moisture content of 12%.

Notes:

1. The tabulated values are for dry service conditions and standard-term duration of load.

2. Nordic Lam ES12/NPG members are symmetrical throughout the depth and the width of the member (homogeneous layups).

3. The tabulated values apply to members consisting of 4 or more laminations, unless otherwise indicated. It should be noted that only 3-1/2 x 3-1/2 inches columns consist of 3 laminations.

4. Design of glulam members shall be in accordance with CSA O86:19. It should be noted that Clause 7.5.3 is not applicable.

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## ES12 Columns – Selection Tables

### Maximum Factored Axial Load, P<sub>f</sub> (lbf)

Effective length – (ft) –		Width = 3-1/2"		Width	= 5-1/2"	Width = 7"
		Depth		De	epth	Depth
	3-1/2"	5-1/2"	7"	5-1/2"	7"	7"
6	17,720	31,345	39,895	65,565	83,445	111,195
7	14,915	25,590	32,565	61,785	78,485	106,715
8	12,345	20,835	26,520	57,335	72,240	101,705
9	10,185	16,990	21,625	51,875	65,325	96,195
10	8,400	13,900	17,690	45,515	57,395	90,270
11	6,940	11,405	14,510	39,870	50,335	84,065
12	5,750	9,390	11,935	34,920	44,130	77,015
13	4,780	7,765	9,860	30,610	38,705	69,670
14	3,990	6,450	8,185	26,860	33,985	62,945
15	-	-	-	23,605	29,880	56,840
16	-	-	-	20,770	26,305	51,325
17	-	-	-	18,305	23,190	46,365
18	-	-	-	16,155	20,475	41,905
19	-	-	-	14,275	18,100	37,895
20	-	-	-	12,640	16,030	34,295
21	-	-	-	11,210	14,220	31,060
22	-	-	-	9,960	12,640	28,155
23	-	-	-	-	-	25,540
24	-	-	-	-	-	23,185

### Factored Bearing Resistances (lbf)

Species combination or stress grade	Bearing area (in.²)						
	12.25	19.25	24.50	30.25	38.50	49.00	
	3-1/2" x 3-1/2"	3-1/2" x 5-1/2"	3-1/2" x 7"	5-1/2" x 5-1/2"	5-1/2" x 7"	7" x 7"	
Douglas Fir-Larch	11,440	17,980	22,885	28,255	35,960	45,770	
Hem-Fir	7,520	11,815	15,040	18,565	23,630	30,075	
Spruce-Pine-Fir	8,665	13,615	17,325	21,395	27,225	34,655	
Northern Species	5,720	8,990	11,440	14,125	17,980	22,885	
Nordic Lam ES11	9,480	14,900	18,960	23,410	29,795	37,920	
Nordic Lam 24F-1.9E	12,260	19,265	24,520	30,275	38,530	49,035	

Notes:

1. For preliminary design use only. Final design shall include a complete analysis including the verification of the factored bearing resistance.

2. The tabulated values are for dry service conditions and standard-term duration of load.

3. Values shown in the table above are the maximum factored axial loads, in pounds (lbf), that can be applied to the column in addition to its own weight. Values shown in the bottom table are the factored bearing resistances, in pounds (lbf).

4. The tabulated factored axial loads are based on simply axially loaded columns subjected to an eccentricity of either 1/6 member width or depth, whichever is worse.

5. The tabulated factored bearing resistances are based on the compression perpendicular to grain resistance of the supporting material.